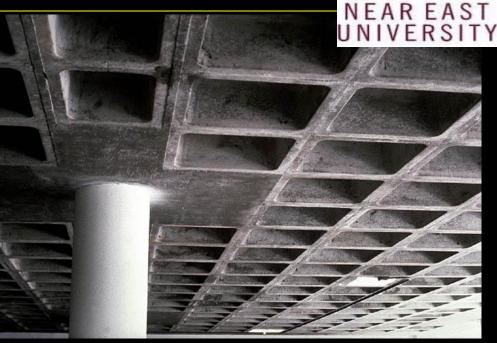
ARCHITECTURAL STRUCTURES:

FORM, BEHAVIOR, AND DESIGN

ARCH 331
HÜDAVERDİ TOZAN
SPRING 2013

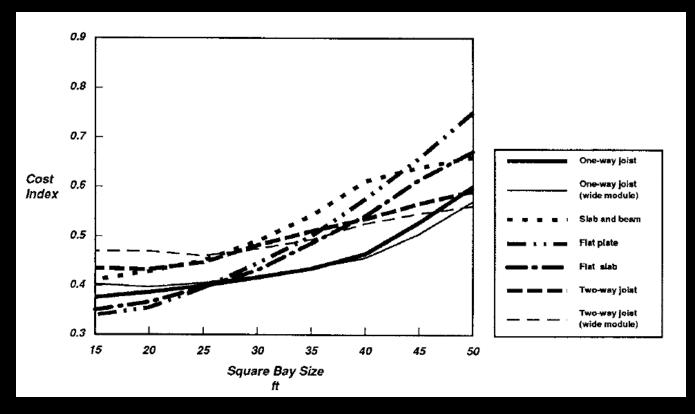
twenty five



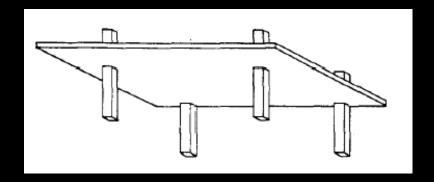
http:// nisee.berkeley.edu/godden

concrete construction: flat spanning systems

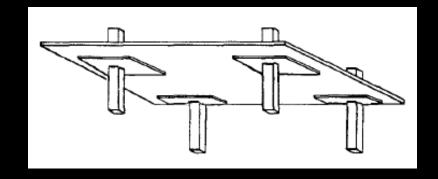
- economical & common
- resist lateral loads



- flat plate
 - 5"-10" thick
 - simple formwork
 - lower story heights

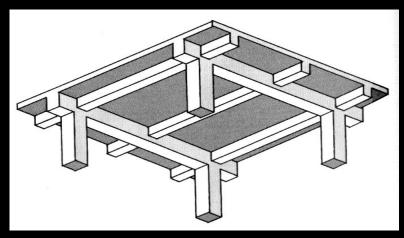


- flat slab
 - same as plate
 - 2 1/4"-8" drop panels

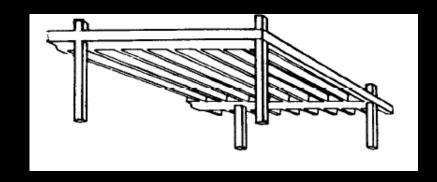


- beam supported
 - slab depth ~ L/20
 - 8"-60" deep

- one-way joists
 - 3"-5" slab
 - 8"-20" stems
 - 5"-7" webs



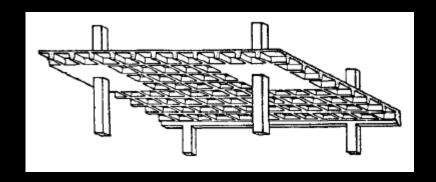
The Architect's Studio Companion

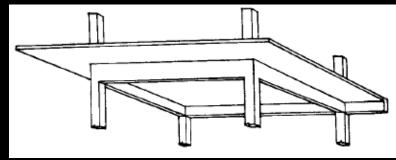


- two-way joist
 - "waffle slab"
 - 3"-5" slab
 - 8"-24" stems
 - -6"-8" webs

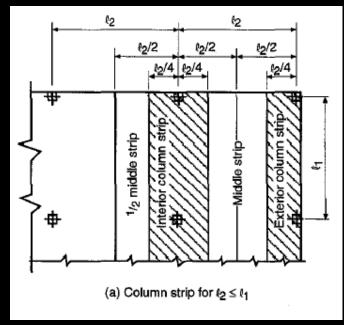


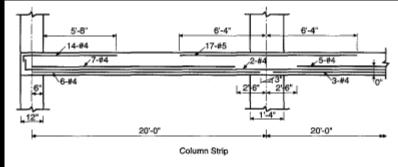
- 5"-10" slabs
- taller story heights





- simplified frame analysis
 - strips, likecontinuous beams
- moments require flexural reinforcement
 - top & bottom
 - both directions of slab
 - continuous, bent or discontinuous





one-way slabs (wide beam design)

- approximate analysis for moment & shear

coefficients

- two or more spans

- ~ same lengths
- w_u from combos
- Uniformly Distributed Load (L/D ≤ 3)

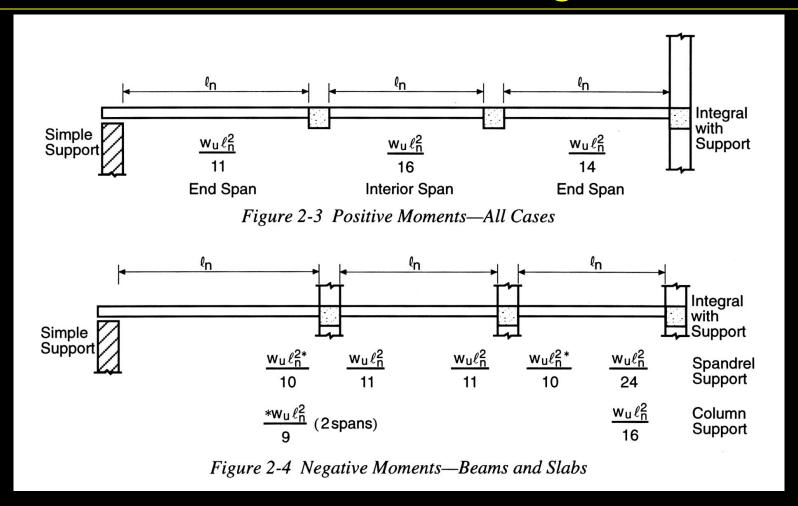
 Solution

 Two or More Spans

 Figure 2-2 Conditions for Analysis by Coefficients (ACI 8.3.3)

uniform loads with L/D ≤ 3

- ℓ_n is clear span (+M) or average of adjacent clear spans (-M)



- two-way slabs Direct Design Method
 - 3 or more spans each way
 - uniform loads with L/D ≤ 2
 - rectangular panels with long/short span ≤ 2
 - successive spans can't differ > longer/3
 - column offset no more than 10% span

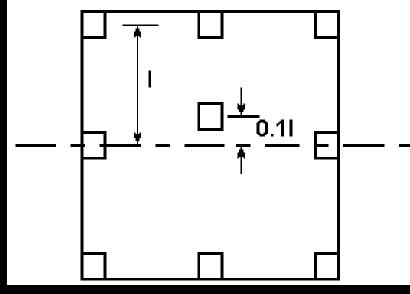
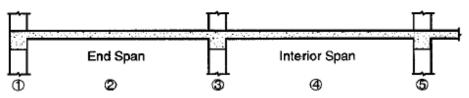


Table 4-6 Two-Way Beam-Supported Slab



		End Span			Interior Span	
		1	2	3	4	5
Span	Slab Moments	Exterior		First Interior		Interior
ratio		Negative	Positive	Negative	Positive	Negative
ર₂/સ	Total Moment	0.16 M _O	0.57 M _o	0.70 M _o	0.35 M _o	0.65 M _O
0.5	Column Strip Beam	0.12 M _o	$0.43\mathrm{M}_\mathrm{O}$	0.54 M _o	0.27 M _o	0.50 M _o
	Slab	0.02 M _O	0.08 M _o	0.09 M _o	0.05 M _o	0.09 M _O
	Middle Strip	0.02 M _O	0.06 M _O	0.07 M _o	0.03 M _o	0.06 M _O
1.0	Column Strip Beam	0.10 M _O	0.37 M _O	0.45 M _o	0.22 M _o	0.42 M _o
	Slab	0.02 M _O	0.06 M _O	0.08 M _O	0.04 M _o	0.07 M _O
	Middle Strip	0.04 M _O	0.14 M _O	0.17 M _o	0.09 M _o	0.16 M _O
2.0	Column Strip Beam	0.06 M _o	0.22 M _O	0.27 M _o	0.14 M _o	0.25 M _Q
	Slab	0.01 M _O	0.04 M _o	0.05 M _o	$0.02 M_{\odot}$	0.04 M _o
	Middle Strip	0.09 M _O	0.31 M _O	0.38 M _O	0.19 M _O	0.36 M _O

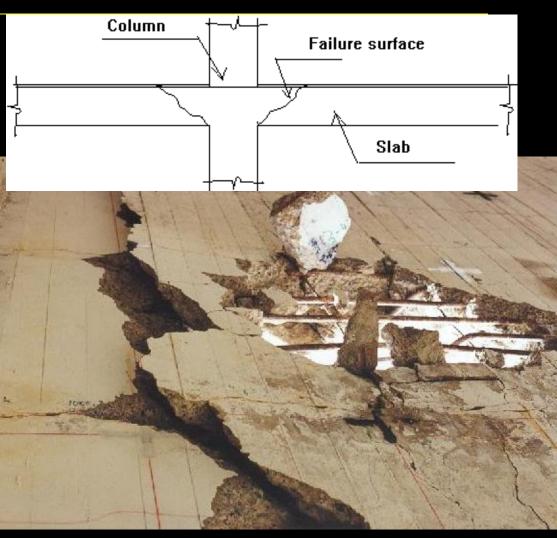
Notes:

- (1) Beams and slab satisfy stiffness criteria: $\alpha_1 \ell_2 / \ell_1 \ge 1.0$ and $\beta_t \ge 2.5$.
- (2) Interpolate between values shown for different &/t1 ratios.
- (3) All negative moments are at face of support.
- (4) Concentrated loads applied directly to beams must be accounted for separately.

Shear in Concrete

- at columns
- want to avoid stirrups
- can use shear studs or heads



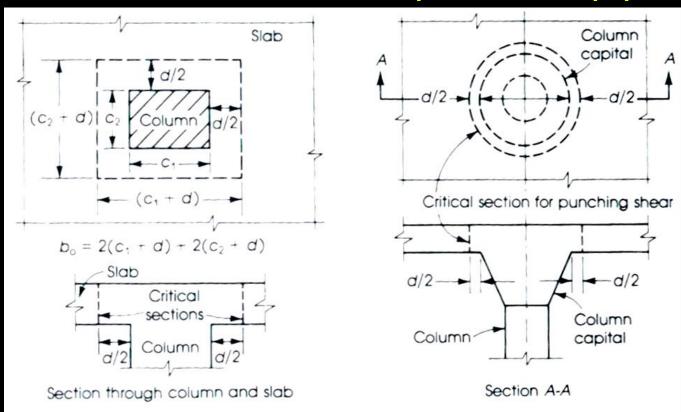


Concrete Spans 11 Lecture 25

Architectural Structures
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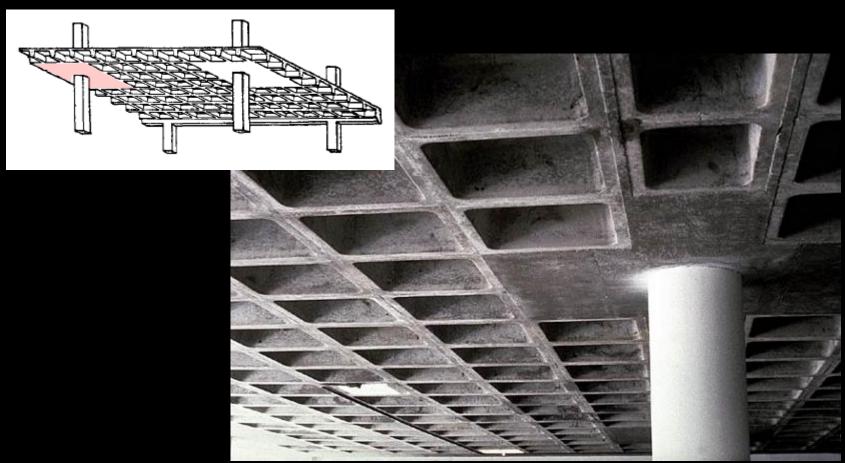
Shear in Concrete

- critical section at d/2 from
 - column face, column capital or drop panel



Shear in Concrete

at columns with waffle slabs



Openings in Slabs

- careful placement of holes
- shear strength reduced
- bending & deflection can increase

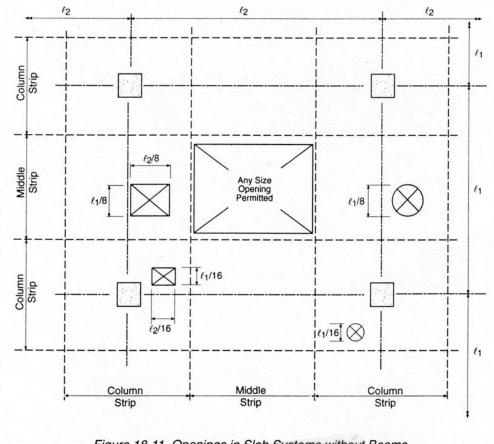
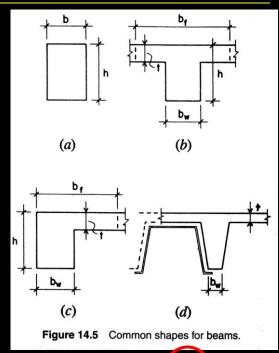


Figure 18-11 Openings in Slab Systems without Beams

General Beam Design

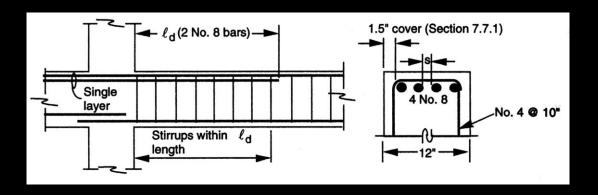
- f_c & f_y needed
- usually size just b & h
 - even inches typical (forms)
 - similar joist to beam depth
 - b:h of 1:1.5-1:2.5
 - $-b_w & b_f$ for T
 - to fit reinforcement + stirrups
- slab design, t
 - deflection control & shear



$$S = \frac{bh^2}{6}$$

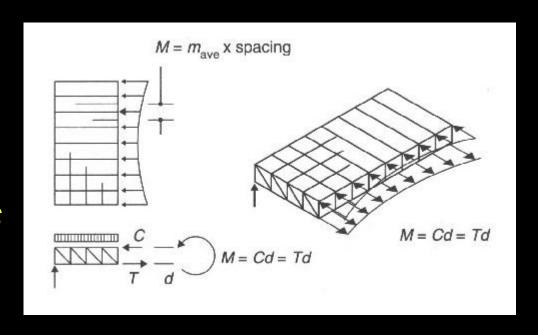
General Beam Design (cont'd)

- custom design:
 - longitudinal steel
 - shear reinforcement
 - detailing



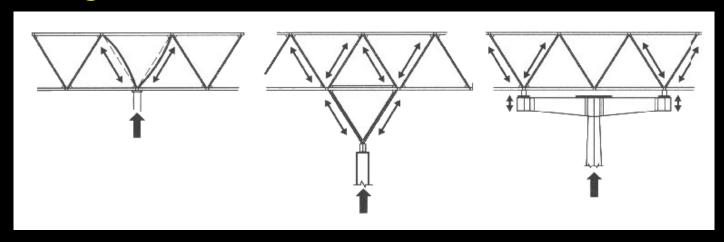
Space "Frame" Behavior

- handle uniformly distributed loads well
- bending moment
 - tension & compression "couple" with depth
 - member sizescan vary,but difficult



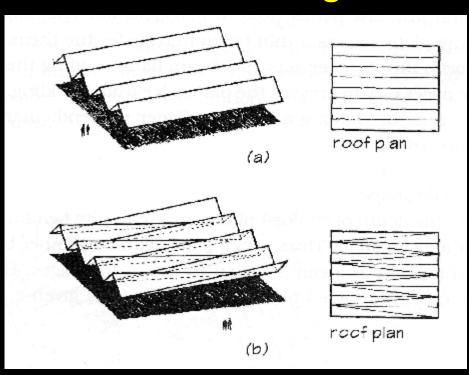
Space "Frame" Behavior

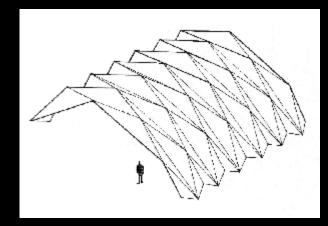
- shear at columns
- support conditions still important
 - point supports not optimal
- fabrication/construction can dominate design

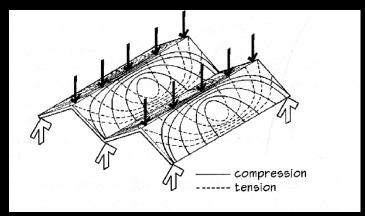


Folded Plates

- increased bending stiffness with folding
- lateral buckling avoided







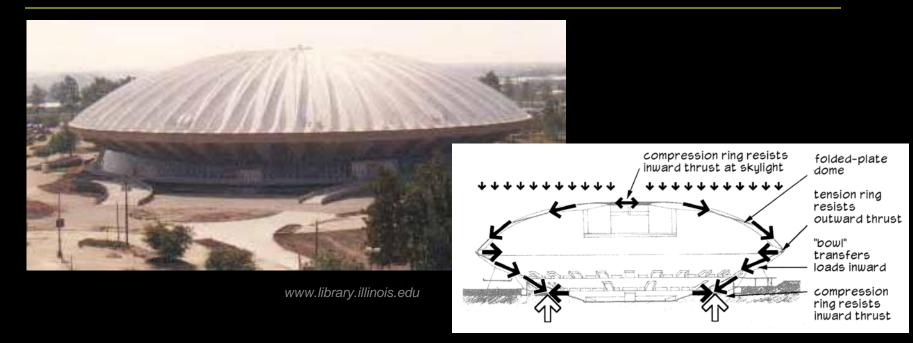
Folded Plates

common for roofs

edges need stiffening



Folded Plates



- Assembly Hall, University of Illinois
- Harrison & Abramovitz 1963
- Edge-supported dome spanning 400 feet wound with 614 miles of one-fifth inch steel wire